Effects of debris flow composition on runout, depositional mechanisms, and deposit morphology in laboratory experiments

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Abstract Predicting debris flow runout is of major importance for hazard mitigation. Apart from topography and volume, runout distance and area depends on debris flow composition and rheology, but how is poorly understood. We experimentally investigated effects of composition on debris flow runout, depositional mechanisms, and deposit geometry. The small-scale experimental debris flows were largely similar to natural debris flows in terms of flow behavior, deposit morphology, grain size sorting, channel width-depth ratio, and runout. Deposit geometry (lobe thickness and width) in our experimental debris flows is largely determined by composition, while the effects of initial conditions of topography (i.e., outflow plain slope and channel slope and width) and volume are negligible. We find a clear optimum in the relations of runout with coarse-material fraction and clay fraction. Increasing coarse-material concentration leads to larger runout. However, excess coarse material results in a large accumulation of coarse debris at the flow front and enhances diffusivity, increasing frontal friction and decreasing runout. Increasing clay content initially enhances runout, but too much clay leads to very viscous flows, reducing runout. Runout increases with channel slope and width, outflow plain slope, debris flow volume, and water fraction. These results imply that debris flow runout depends at least as much on composition as on topography. This study improves understanding of the effects of debris flow composition on runout and may aid future debris flow hazard assessments.

1. Introduction

Debris flows are common phenomena in mountainous regions. They differ from rock avalanches and sediment-laden water floods because both solid and fluid forces influence their motion and govern their rheological properties [Costa, 1988; Iverson, 1997]. Typically, debris flows contain 20% to 60% water by volume [Costa, 1988; Pierson, 2005], peak velocities can surpass 10 m/s, and large flows can exceed 1 km³ [e.g., Iverson, 1997]. They denude mountainsides, inundate channels, floodplains, and alluvial fans, and thereby present a major hazard for people and structures [e.g., Jakob, 2005]. The socioeconomic impacts of debris flows may grow with the current increase of landscape exploitation and extreme precipitation events and permafrost degradation forced by global warming [e.g., Rebetz et al., 1997; Jakob and Friele, 2010; Stoffel et al., 2014].

The assessment of runout distance and area is critical for delineating areas at risk from debris flows [D’Agostino et al., 2010]. Several methods have been proposed to predict debris flow runout [e.g., Rickenmann, 2005; D’Agostino et al., 2010; Griswold and Iverson, 2008; Scheidl et al., 2013]. These methods depend mainly on topography (i.e., the slope, width, and length of the upstream channel and the slope of the outflow plain) and debris flow volume [e.g., Takahashi, 1991; Bathurst et al., 1997; Rickenmann, 1999; Berti and Simon, 2007]. However, debris flow composition has often been neglected for practical reasons, although it has a profound effect on debris flow dynamics and runout distance [e.g., Whipple and Dunne, 1992; Scheidl and Rickenmann, 2010; Hürlimann et al., 2015].

Debris flow composition is commonly neglected because it is very difficult to constrain material properties from debris flow deposits in the field [e.g., Iverson et al., 2010]. Natural debris flows are monitored in a few high-frequency debris flow torrents [e.g., Hürlimann et al., 2003; Takahashi, 2009; Zhou and Ng, 2010; McCoy et al., 2010; Marchi and Tecca, 2013], but monitoring faces many difficulties because of the rapid, short-lasting, infrequent, and destructive nature of debris flows. Debris flows cause the greatest threat when they move
Debris flow composition strongly affects deposit geometry, such as the height and width of levees and lobes [e.g., Whipple and Dunne, 1992; Coussot et al., 1998; Major and Iverson, 1999]. However, the relation between debris flow composition and deposit geometry (i.e., lobe and levee thickness and width) is still poorly understood.

We use experiments to investigate the effects of debris flow composition on runout distance and deposit geometry. Several authors have previously used laboratory flumes to simulate debris flows [e.g., Van Steijn and Coutard, 1989; Liu, 1996; Major and Iverson, 1999; D’Agostino et al., 2010; Hürlimann et al., 2015]. Well-developed unchanneled experimental debris flows with self-formed levees and a marked depositional lobe have only been produced in the large-scale U.S. Geological Survey (USGS) laboratory flume [e.g., Iverson et al., 2010] but not in smaller-scale flumes. Small-scale experiments are useful because they allow experiments to be done in large numbers and for a wide range of compositions. Iverson and Denlinger [2001] and Iverson et al. [2010] suggested that dynamic similarity between natural and small-scale debris flows is probably unattainable, as small-scale debris flows are likely to show disproportionately large effects of yield strength, viscous flow resistance, and grain inertia, while showing a disproportionately small effect of pore fluid pressure. Nonetheless, despite these scale effects, we managed to create small experimental debris flows that show similar flow behavior, grain segregation, runout, and deposit morphology as natural debris flows [e.g., Blair, 1999; Johnson et al., 2012].

Here we experimentally investigate effects of debris flow composition on debris flow runout distance and area, depositional mechanisms, and deposit geometry. This paper is structured as follows. We first detail layout and boundary conditions of the experimental flume and laboratory experiments. Then we detail measurement techniques and dimensionless parameters used to evaluate flow regimes in the experimental debris flows and to address scaling. We present observations of the morphology and sediment sorting and infer flow regimes and effects of debris flow composition, volume, and topography on runout distance and deposit morphology. Finally, we discuss effects of debris flow composition on depositional mechanisms, runout distance, and deposit geometry and elaborate on the scaling of debris flows in small-scale laboratory flumes.

2. Methods
2.1. Methodology
We conducted a series of small-scale debris flow experiments with systematic variations of angular gravel (2–5 mm), clay (kaolinite), and water fractions relative to a reference debris flow mixture, consisting of gravel, sand, and clay mixed with water (Tables 1 and S1 in the supporting information). The gravel and clay fractions are defined as the fraction within the total solids volume, and the water fraction is defined as the volume of water relative to the total debris flow volume (solids and water). For simplicity, we converted mass to volume by assuming a constant solids density of 2650 kg/m$^3$, which might introduce a small volume underestimation for the debris flows rich in basaltic gravel (basalt density $\approx$3400 kg/m$^3$; underestimation $\sim$2% for most debris flows and up to 9% for the most gravel-rich debris flow). We also tested the topographic effects of channel width, channel length, channel slope, and outflow plain slope. Finally, the effect of outflow plain composition was tested, by using an initial unconsolidated $\sim$1 cm thick bed of sand, a fixed rough bed (sand glued to a plate), and a fixed smooth bed (plastic). After initial tests we selected a reference sediment mixture (see section 2.3) for all experiments and gravel, clay, and water fractions were systematically varied relative to this mixture. We found that experiments were repeatable, although natural variability caused considerable variations in some debris flows. To account for the effects of natural variability, we generally conducted each experimental setting at least 3 times. Using photograph, video, and digital elevation model (DEM) analyses, we mapped the dynamics, morphology, and sediment sorting of the debris flows. This data documented flow velocity and flow depth of the debris flows during motion and the runout distance and area, lobe width, lobe height, and levee height of deposits.

2.2. Experimental Setup and Data Collection
The experimental flume consisted of a straight, rectangular, channel of 2 m long and 12 cm wide (Figure 1), connected to an unconstrained lower angle outflow plain. Upstream, the channel was connected to a manual mixing tank with a gate that opened electromagnetically for rapid release of well-mixed debris. Sediment and water were agitated in the mixing tank for $\sim$20 s, and agitation stopped simultaneously with gate opening.
Table 1. Varied Debris Flow Composition and Topographic Parameters

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Unit</th>
<th>Reference</th>
<th>Range</th>
<th>Number of Experiments</th>
</tr>
</thead>
<tbody>
<tr>
<td>Debris Flow Composition</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Gravel fraction</td>
<td>g</td>
<td>900</td>
<td>0–3624</td>
<td>47</td>
</tr>
<tr>
<td></td>
<td>vol %</td>
<td>18</td>
<td>0–72</td>
<td></td>
</tr>
<tr>
<td></td>
<td>wt %</td>
<td>14</td>
<td>0–56</td>
<td></td>
</tr>
<tr>
<td>Clay fraction</td>
<td>g</td>
<td>100</td>
<td>0–1899</td>
<td>50</td>
</tr>
<tr>
<td></td>
<td>vol %</td>
<td>2.0</td>
<td>0–38</td>
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<tr>
<td></td>
<td>wt %</td>
<td>1.5</td>
<td>0–29</td>
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<tr>
<td>Water fraction</td>
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<td>1200–2500</td>
<td>33</td>
</tr>
<tr>
<td></td>
<td>vol %</td>
<td>44</td>
<td>39–57</td>
<td></td>
</tr>
<tr>
<td></td>
<td>wt %</td>
<td>23</td>
<td>19–33</td>
<td></td>
</tr>
<tr>
<td>Total mass</td>
<td>g</td>
<td>6500</td>
<td>1950–11050</td>
<td>27</td>
</tr>
<tr>
<td>Topography</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Channel slope</td>
<td>deg</td>
<td>30</td>
<td>22–34</td>
<td>41</td>
</tr>
<tr>
<td>Outflow plain slope</td>
<td>deg</td>
<td>10</td>
<td>0–15</td>
<td>29</td>
</tr>
<tr>
<td>Channel width</td>
<td>cm</td>
<td>12</td>
<td>4.5–12</td>
<td>16</td>
</tr>
<tr>
<td>Channel length</td>
<td>m</td>
<td>2</td>
<td>2–3</td>
<td>16</td>
</tr>
</tbody>
</table>

*Note that the bulk of the debris flow mixture consisted of sand, but this was not systematically varied and therefore not shown in this table.

*Similar series.

The gate opened by swinging upward. Relatively soft tap water was used in the experiments (5.4° DH). To simulate natural bed roughness, the channel bed and sidewalls were covered with sandpaper (grade 80), whereas the outflow plain was covered by a layer of ~1 cm unconsolidated reference mixture (without water and clay).

In the channel floor a hatch was present 76 cm upstream of the intersection point of channel and outflow plain. This hatch was opened at a constant time interval (1.5 s) after release of debris from the mixing tank, to divert the debris flow tail and prevent it from filling the initial channel and obscuring sorting patterns. A similar approach was followed in debris flow experiments in the USGS flume by Johnson et al. [2012]. Truncation of the tail did not affect runout or lobe thickness as the dilute tail mainly backfilled the channel in between the levees. The estimated volume of the truncated tail is 10–20% of the total volume at maximum.

![Figure 1. Experimental flume setup. (a) Photograph. (b) Schematic overview.](image-url)
Multiple cameras were used to photograph debris flow deposits and capture debris flow motion. Debris flow deposits were photographed with a Canon PowerShot A640 camera suspended above the flume. Debris flow velocity was inferred from movies shot with a Canon Powershot A650 IS camera. Flow depth was measured near the channel apex, from a movie shot with a Canon Powershot A650 IS that captured a tape measure (accuracy \( \sim 2-3 \text{ mm} \)). Flow depth was only measured in the last 58 experiments.

Deposit morphology was measured with a Vialux z-Snapper 3-D scanner that captured a high-accuracy 3-D point cloud from a fringe pattern projector and camera (submillimeter vertical and horizontal accuracy) [Hoefling, 2004]. Point clouds from the 3-D scanner were processed with MATLAB (The MathWorks, version 7.13.0.564) using natural neighbor interpolation to a gridded DEM of 1 mm resolution. The DEM was used for visualization and to measure runout distance and area, lobe width, lobe height, and levee height (Figure 2). Runout distance was defined as the distance from the apex to the maximum extent of the debris flow. Deposit area was defined as the total area of the debris flow deposit on the outflow plain. We characterized lobe height as its maximum value and lobe width as the point where its sides became approximately parallel, upstream from its maximum extent. Levee height was measured where levees were well developed, sufficiently downstream of the apex and upstream of the lobe terminus. In cases where levee height varied considerably, we averaged over multiple measurements along the flow deposit.

2.3. Debris Flow Composition

The debris flow mixtures were composed of four basic sediments, combined in different ratios. These were clay (kaolinite), well-sorted fine sand, poorly sorted coarse sand, and basaltic gravel (2–5 mm) (Figure 3). The bulk of the mixture generally consisted of sand (Figure 3c). The dark-toned gravel conveniently highlighted textural patterns within the debris flows. The gravel in our experimental debris flows behaved similar to the coarse-grained fraction (typically cobble- to boulder-sized debris) in natural debris flows, in the sense that it formed the coarse-grained flow front and levees. To clarify discussion and comparison between experimental results and nature, we therefore use coarse-grained fraction and gravel fraction as synonyms throughout the text.

The reference sediment mixture consisted of 100 g clay, 1050 g fine sand, 2950 g coarse sand, and 900 g gravel, mixed with 1500 g of water (Figure 3a). We varied the fractions of gravel and clay in the sediment mixture (Figure 3b) but kept the total amount of sediment constant (5000 g). Debris flow volume only varied in the set of experiments where volume was explicitly varied and in one set of experiments wherein the amount of water was varied to evaluate the effect of water to sediment ratio (Table 1).
2.4. Geotechnical Properties

In independent tests we measured the permeability and diffusivity of the debris flow mixtures. As these measurements were performed in independent tests, they represent the characteristics of the sediment after release from the mixing tank before grain size segregation occurred.

Permeability of the debris flow mixtures was measured with a constant head permeameter [e.g., Klute and Dirksen, 2003] (see also the American Society for Testing and Materials D2434 method procedure). A similar amount of sediment and water as used in the experiments was mixed and then poured in a crate, with a permeable mesh at the bottom and sides to stimulate drainage. After 20 min, five samples were obtained from the deposits for analysis in the permeameter.

Diffusivity was measured with the method of Major [2000]. We used a smooth-walled, 0.125 m diameter transparent tube in vertical position with an impermeable floor, which we filled to a height of approximately 0.55 m with a well-mixed sediment mixture. Pore fluid pressures were measured at 0.05, 0.15, 0.25, and 0.45 m above the bed. Excess fluid pressure was determined by subtracting hydrostatic fluid pressure from measured total fluid pressure. Hydrostatic fluid pressure $P_h$ (Pa) was calculated as

$$P_h = \rho_f g (h - z)$$

where $z$ is height above the bed (m), $h$ is deposit thickness (m), and $g$ is acceleration due to gravity (9.81 m/s$^2$).

The mass density of the interstitial fluid $\rho_f$ is defined as the mass densities of pure water $\rho_w$ (1000 kg/m$^3$) and fines (silt and clay) combined ($\rho_f$, assumed 2650 kg/m$^3$) [cf. Iverson, 1997]:

$$\rho_f = \rho_f \text{v}_{\text{fines}} + \rho_w (1 - \text{v}_{\text{fines}})$$

where $\text{v}_{\text{fines}}$ is the volume fraction of the interstitial fluid occupied by fines. The decrease of excess fluid pressure as fluid moves out of the mixture and sediment settles is described by the following relation [Major, 2000]:

$$P_* = 8P_{*0} \sum_{n=0}^{\infty} \frac{1}{(2n + 1)^2\pi^2} \cos(\lambda_n z) e^{-\lambda_n^2 D t}$$

where $D$ is the diffusion coefficient (m$^2$/s), $P_*$ is the measured excess fluid pressure (Pa) at $z$, and $t$ is time of measurement relative to start of the experiment (s). The eigenvalues $\lambda_n$ are defined as

$$\lambda_n = \frac{(2n + 1)\pi}{2h}$$

$P_{*0}$ represents the initial excess pore fluid pressure (Pa) at $z = 0$ in a fully saturated mixture:

$$P_{*0} = (\rho_s - \rho_f) (1 - \text{v}_f) g (h - z)$$

where $\rho_s$ is the mass density of solid particles (2650 kg/m$^3$). The volumetric interstitial fluid fraction $\text{v}_f$ is the combined volumetric fraction of pure water fraction and fines (silt and clay), which equals the porosity in a
saturated mixture [Iverson, 1997]. We empirically determined the value of the diffusion coefficient $D$ by iteratively minimizing the difference between measured and predicted excess fluid pressure averaged over all depths [cf. Major, 2000].

Geotechnical properties were not measured for every debris flow sediment composition used. For the calculation of dimensionless parameters denoting flow regime and scaling (sections 2.5 and 2.6) we fitted (linearly or exponentially, depending on the data trend) the missing values based on measured values. The same was done for missing values of flow depth.

### 2.5. Dimensionless Characterization of Flow Regime

Iverson [1997], Iverson and Denlinger [2001], Iverson et al. [2010], and Iverson [2015] present a set of dimensionless parameters to describe debris flow regime. These allow comparisons between debris flows of different sizes and scales and enable quantitative assessment of the similarity in flow regime of experimental to natural debris flows. Here we summarize these parameters.

There are three forces that resist motion in debris flows: collisional, frictional, and viscous forces [Iverson, 1997; Parsons et al., 2001; Iverson and Denlinger, 2001]. Three dimensionless parameters describe the relationship between these forces. The Bagnold number defines the ratio of collisional to viscous forces:

$$N_B = \frac{\nu_s \rho_s \delta^2 \gamma}{(1 - \nu_s) \mu}$$

wherein $\delta$ is the mean grain size of a debris flow mixture (m) [cf. Iverson, 1997], $\nu_s$ is the volumetric solids fraction and $\gamma$ is the flow shear rate (1/s):

$$\gamma = \frac{u}{H}$$

wherein $u$ is flow velocity (m/s) and $H$ is flow depth (m). We estimate the interstitial fluid viscosity $\mu$ as [Thomas, 1965, cf. Iverson, 1997]

$$\frac{\mu}{\mu_w} = 1 + 2.5 \nu_{fines}^2 + 10.05 \nu_{fines}^2 + 0.00273 \exp(16.6 \nu_{fines})$$

where $\mu_w$ is the dynamic viscosity of pure water (0.001002 Pa s).

The ratio of collisional to frictional forces is defined by the Savage number:

$$N_S = \frac{\rho_s \delta^2 \gamma^2}{(\rho_s - \rho_f) g H \tan \phi}$$

wherein $\phi$ is the internal angle of friction (assumed 42°) [cf. Parsons et al., 2001]. The ratio of frictional to viscous forces is defined by the friction number:

$$N_F = \frac{\nu_s (\rho_s - \rho_f) g H \tan \phi}{(1 - \nu_s) \gamma \mu}$$

Iverson [1997] constrains the magnitudes at which these dimensionless parameters transition from one force being dominant over the other, mostly based on experiments of cohesionless dry flows with unimodal spherical particles [Bagnold, 1954; Savage and Hutter, 1989]: collisional forces dominate over viscous forces for $N_B > 200$, collisional forces dominate over frictional forces for $N_S > 0.1$, and frictional forces dominate over viscous forces for $N_F > 2000$. In contrast, experimental data of water-saturated small-scale debris flows of Parsons et al. [2001] suggest that frictional forces start dominating viscous forces at $N_F > 100$ for the flow body and $N_F > 250$ for the flow front. We test both transitions against our data in section 3.2.

The ratio of solid inertia to fluid inertia is described by the mass number [Iverson, 1997]:

$$N_M = \frac{\nu_s \rho_s}{(1 - \nu_s) \rho_f}$$

Although no experimental data on the transition values of $N_M$ are available, its qualitative influence is obvious from its definition: grain inertia becomes unimportant as the density or concentration of grains approaches zero. Natural grain density has a relatively narrow range compared to other variables, however. The effect of the proportion of water in the flow will be assessed experimentally.
The Darcy number \( (N_D) \) describes the tendency for pore fluid pressure to buffer grain interactions:

\[
N_D = \frac{\mu}{\nu_r \rho_s \gamma k}
\]  
(12)

where \( k \) is permeability (m\(^2\)). Iverson and LaHusen [1989] reported experiments with \( 1000 < N_D < 6000 \), in which large fluid pressure fluctuations evidenced strong solid-fluid interactions. Following Iverson [1997], we assume that values of \( N_D \) in this range apply to debris flows.

The grain Reynolds number compares the effects of particle collisions and pore fluid viscosity [Iverson, 1997]. It defines the ratio between the solid inertial stress and the fluid viscous shearing stress:

\[
N_{Rg} = \frac{N_B}{N_M} = \frac{\rho f \gamma \delta^2}{\mu}
\]  
(13)

Typically, fluid flow with respect to grains begins to show inertial effects and deviates significantly from ideal viscous behavior for \( N_{Rg} > 1 \) [Vanoni, 1975].

### 2.6. Dimensionless Characterization of Scaling

Iverson and Denlinger [2001], Savage and Iverson [2003], and Iverson et al. [2010] show how scale-dependent behavior, which potentially causes scale effects in small experiments, can be assessed by several dimensionless parameters. The parameter \( N_R \) provides a measure of the influence of viscous effects relative to flow size:

\[
N_R = \frac{\rho H \sqrt{gL}}{\mu}
\]  
(14)

where \( \rho \) is the mass density of the debris flow mixture (here about 1920 kg/m\(^3\)) and \( L \) is the maximum length of the flow mass (m), here assumed equal to the channel length. Viscous effects will be less important in large flow (i.e., those with large \( H \sqrt{gL} \)) than in small flows with the same dynamic viscosity \( \mu \) [Iverson and Denlinger, 2001].

The parameter \( N_p \) expresses the ratio of timescales for debris flow motion and pore pressure diffusion:

\[
N_p = \frac{\sqrt{L/g}}{H/P/D}
\]  
(15)

Values of \( N_p << 1 \) apply in most geophysical flows and indicate that if high pore pressure (i.e., excess pore pressure) develops, it persists much longer than the time needed for downslope grain flow motion. Moreover, because \( N_p \) decreases quadratically as the flow thickness \( H \) increases, large-scale flows preserve high pore pressures much longer than small-scale flows with the same mixture composition. The difference in pore pressure diffusion between large-scale and small-scale debris flows may therefore affect flow dynamics [Iverson and Denlinger, 2001].

### 3. Results

In this section we first describe flow behavior, deposit morphology, and sediment sorting in the experimental debris flows with various compositions. Then flow regimes of the debris flows with various compositions are discussed in terms of dimensionless numbers. Next, we identify effects of debris flow composition and initial conditions of debris flow volume and topography on runout distance and deposit morphology and geometry. We evaluate the effect of flow momentum on runout and deposit geometry. Finally, we assess the effect of outflow plain bed composition.

#### 3.1. Flow Behavior, Deposit Morphology, and Sediment Sorting

Multiple flow surges were observed in the debris flows, and coarse particles were observed to concentrate at the flow front in the majority of experimental debris flows (Movies S1–S8 in the supporting information). When entering the outflow plain, the flow front was continuously shouldered aside into lateral levees. The levees then laterally confined the flow to form elongate debris flow deposits. Segregation into more resistive gravelly flow fronts and finer-grained, more dilute, tails was observed in these debris flows. Only in debris flows with a very high clay fraction (>0.22), where viscous forces increasingly dominated over collisional and frictional forces, did coarse particles not accumulate at the flow front. In those flows levees did not form, which caused the debris flows to spread laterally after leaving the channel (Movie S5).
Figure 4. Morphology and sediment sorting of selected experimental debris flows. (a–c) Oblique photographs of debris flows with decreasing gravel content. The coarse-grained frontal accumulation decreases with decreasing gravel concentration, and grain size segregation becomes less evident. (d–f) Oblique photographs of debris flows with increasing clay content. Grain size segregation decreases with increasing clay content and eventually is inhibited when flows become viscous (Figure 4f). (g and h) Cross sections highlighting particle size difference between levees and channel. (i–l) Top view photographs highlighting changes in debris flow runout and texture with decreasing gravel content and increasing clay content. $F_g$ refers to vol % of gravel, and $F_c$ refers to vol % of clay. Details on experimental debris flows are in Table S1.
Figure 5. Hillshade images of selected debris flows. (a–c) Variable gravel content: 0, 23, and 49 vol % gravel, respectively. An initial increase in gravel content enhances runout, but above an optimum value runout is reduced. (d–f) Variable clay content: 0, 11, and 22 vol % clay, respectively. Adding clay to the debris flows initially enhances runout, but above a threshold of ~22 vol % runout decreases dramatically. (g–i) Variable water content: 41, 44, and 51 vol %, respectively. Increasing water content enhances runout. (j–l) Volume variation: 0.0020, 0.0034, and 0.0051 m³, respectively. Increasing debris flow volume enhances runout. (m–o) Variable channel slope: 24°, 28°, and 34°, respectively. Debris flows run out longer on steeper channel slopes. (p–r) Variable outflow plain slope: 0°, 10°, and 15°, respectively. Debris flow runout is larger on steeper outflow plain slopes. (s–u) Channel width variation: 4.5, 9.5, and 12 cm wide, respectively. An increase in channel width from 4.5 to 12 cm increases debris flow runout. (v) Smooth and rigid outflow plain bed. Long and wide runout, a thin deposit and no levees and grain size segregation. (w) No diversion of debris flow tail results in filling of the leveed channel and in a few cases a little overflow of the channel. (x) Longer channel, 3 m instead of 2 m. A longer channel does not discernably influence runout and deposit morphology. Figures 5v–5x can be compared to reference experiments shown in Figures 5h, 5k, and 5q, which have similar composition.

Under a wide range of conditions debris flows formed deposits that consisted of a channel bordered by self-formed lateral levees. These ended in a well-defined depositional lobe (Figures 4 and 5), with coarse gravel particles concentrated in lateral levees and at lobe margins. Thus, in most deposits there was a marked difference in particle size between the deposit margins and interior. Deposit interiors contained much finer particles, both at the surface (Figures 4a, 4b, 4d, and 4i) and in cross section (Figure 4g). Similar, although less pronounced, sorting patterns were present in debris flows with a relatively high clay fraction (Figures 4e, 4h, and 4k). However, in debris flows for which the clay fraction exceeded 0.22, distinct particle size sorting
Figure 6. Flow regime of experiments with varying gravel, clay, and water fraction. Data points are experimental results averaged per parameter. The majority of the experimental debris flows was observed to have a frictional flow regime given the effective grain size segregation [Vallance and Savage, 2000]. Flows with a clay fraction $>0.22$ had a viscous flow regime. The Parsons et al. [2001] boundary in Figures 6g–6i between dominant frictional and viscous forces is thus more accurate for our experimental debris flows than the boundaries proposed by Bagnold [1954] and Savage and Hutter [1989]. The exceptionally large Savage number for a clay fraction of 0.20 in Figure 6e results from a relatively large shear rate, caused by relatively high flow velocity and low flow depth. Reference experiments are indicated by gray fill.

was absent (Figure 4f). This was probably caused by the high viscosity of these debris flows, wherein grain interactions are effectively buffered by a highly viscous pore fluid (see section 3.2).

3.2. Flow Regime
Accumulations of coarse particles in lateral levees and frontal margins were observed in all debris flow deposits, except for those with clay fractions exceeding 0.22. Grain size segregation occurs in frictional flows but is inhibited in collisional or viscous flows [Vallance and Savage, 2000]. Frictional grain interactions promote grain size segregation, whereas diffusive mixing hampers segregation in collisional flows and the damped turbulence inhibits percolation and thus segregation in viscous flows [Vallance and Savage, 2000]. This implies...
Figure 7. Flow, morphological, and geotechnical properties as a function of gravel fraction in otherwise the same conditions. The solid line connects the mean values calculated for each gravel fraction class. Reference experiments are indicated by gray fill.

that the experimental debris flows were frictional flows, except for flows with clay fractions >0.22, which behaved as viscous flows.

Most experiments, except those with the highest clay fractions, plot above the transition from viscous to collisional flow regimes and the transition from frictional to collisional flow regimes proposed by Bagnold [1954] and Savage and Hutter [1989] (Figures 6a–6f). However, observations from debris flow deposits imply that frictional forces dominated over collisional forces in these experiments. This suggests that the boundaries proposed by Bagnold [1954] and Savage and Hutter [1989], which are based on dry flow experiments and applied to debris flows by Iverson [1997], are not applicable to our experimental debris flows. This is further supported by the boundaries proposed for the transition between viscous and frictional flow regimes (Figures 6g–6i). Here the transition proposed by Bagnold [1954] and Savage and Hutter [1989] erroneously suggests that the vast majority of the flows had a viscous flow regime. Only the debris flows with clay fractions >0.22 had a viscous flow regime. This transition coincides with the transition proposed by Parsons et al. [2001], which is based on debris flow experiments. The boundary proposed by Parsons et al. [2001] thus more accurately describes the flow regime of our experimental debris flows. These results suggest that the boundaries between viscous, frictional, and collisional flow regimes vary significantly between dry grain flows and water-saturated debris flows.

In debris flows with high clay fractions the effect of fluid inertia outweighs grain inertia (Figures 6k and 6q) and grain interactions become more effectively buffered by viscous pore fluid (Figure 6n), which causes grain inertial effects to decrease (Figure 6q). This is due to the suspension of clay in the pore water, which enhances pore fluid viscosity. A higher water fraction results in a slight decrease in the effect of collisional forces relative to viscous and frictional forces (Figures 6c and 6f). The effect of frictional forces becomes more important relative to viscous forces with increasing water content (Figure 6i), and fluid inertia becomes more important relative to grain inertia (Figure 6l). The relative influence of grain inertial effects increases with increasing gravel concentration (Figure 6p).
3.3. Effects of Debris Flow Composition on Runout Distance and Deposit Morphology

3.3.1. Effects of Coarse-Grained Fraction

Runout distance varied between 0.2 and 1.2 m, for gravel fractions in the debris flows ranging from 0 to 0.72 (Figures 7 and 5a–5c and Movies S1–S3). The longest runout distance occurred for intermediate gravel fractions between 0.25 and 0.5 (Figure 7a), whereas both lower and higher gravel fractions led to shorter runout distances. A similar trend was observed for deposit area, which was also largest for intermediate gravel fractions (Figure 7b). We observed that the optimum runout was related to grain size segregation: at low gravel concentrations levees were insignificant, which allowed lateral spreading and reduced longitudinal runout, whereas high gravel concentrations increased gravel accumulation at the flow front, which reduced runout distance, probably due to increased frictional resistance at the flow front (Figures 4a–4d, 4i, 4j, and 4l).

Flow momentum influenced runout distance and area. The highest flow velocities occurred at the optimal gravel fractions but were lower for both lower and higher gravel fractions (Figure 7b). The decrease in velocity for high gravel fractions was likely caused by the high frictional resistance in the coarse-grained flow front and possibly by increased pore fluid loss due to higher diffusivity (Figure 7g). At low gravel fractions driving collisional forces were probably low, leading to a relatively low flow velocity (Figure 6a).

Lobe height and width were also strongly determined by gravel fraction and resultant grain size segregation (Figures 7e and 7f). Larger gravel concentrations led to larger accumulations of gravel at the terminal flow margins and consequently higher lobes. Low gravel concentrations led to wide lobes because levees could not form and confine the flow. The trend for levee height was similar to but less pronounced than the trend for lobe height versus gravel fraction (Figure 7d).

3.3.2. Effects of Clay Fraction

Clay fraction varied from 0 to 0.38 in the experimental debris flows (Figures 8 and 5d–5f and Movies S1, S4, and S5). Maximum runout distance was 1.45 m, and a clear runout optimum occurred for debris flows with a clay fraction between 0.05 and 0.20 (Figures 8a and 8b). The largest runout distances and area coincide with the largest flow velocities (Figure 8c). The increase in runout and flow velocity for an increase in clay fractions from 0 to 0.2 probably results from increasingly well retained excess pore pressures (Figure 8g).
The highly mobile flows with a clay fraction between 0.05 and 0.2 had a relatively dilute appearance (Movie S5); suspending a small fraction of clay in the pore fluid appears to lubricate the flow. Clay fractions > 0.22 resulted in viscous flows (Figures 6b, 6e, and 4f), which strongly reduced flow velocity and runout. Moreover, grain size segregation is inhibited in viscous flows, reducing the tendency to form levees that would otherwise increase runout distance. Debris flows with a clay fraction of 0.38 were unable to reach the end of the channel.

Lobe height was inversely proportional to runout (Figure 8e), potentially because longer runout allows less bulking of sediment behind the lobe front as the flows had similar volume. Lobe width, on the other hand, was approximately constant over the entire range of clay fractions (Figure 8f). Levee height strongly decreased for increasing clay fraction (Figure 8d), most likely because levee formation is increasingly inhibited in more viscous flows.

Increasing clay fraction reduced permeability and diffusivity (Figure 8g), preventing pore fluid from escaping the mixture. As a result, we visually observed that debris flow deposits with high clay fractions retained excess pore fluid pressure for long times and needed a long time to consolidate.
3.3.3. Effects of Water Fraction

Volumetric water fraction in the debris flows varied from 0.39 to 0.57 (Figures 9 and 5g–5i and Movies S1, S6, and S7). Debris flows with a volumetric water fraction of 0.39 or lower were unable to flow to the end of the channel. As pores need to be completely filled with interstitial fluid to cause excess pore fluid pressure, 0.39 possibly approaches the dynamic porosity of the debris flow mixture. A large water fraction led to an increase in runout and flow velocity and thinner deposits. In general, lobe and levee height decreased with increasing water fraction. Lobe width was approximately constant, although width was larger at relatively low and high water fractions (Figures 9f and 5g–5i).

The debris flows were very sensitive to water content: a 10% increase in volumetric water fraction made the difference between no runout on the outflow plain to over 1.2 m runout. The effect of increasing water fraction was slightly enhanced owing to a minor increase in debris flow volume (i.e., water volume was increased and total amount of sediment was kept constant).

3.4. Effects of Initial Conditions of Flow Volume and Topography

Debris flow volume varied between 0.0010 and 0.0058 m$^3$ (Figures 10 and 5j–5l). An increase in debris flow volume led to a distinct, nearly linear, increase in runout distance and area. Flow velocity also increased with...
enhanced volume (Figure 10b). The effect of flow volume on levee height, lobe height, and lobe width was negligible (Figures 10c–10e).

Outflow plain slope varied between 0° and 15° (Figures 11c–11e). An increase in outflow plain slope resulted in an increase in runout distance and area, but its effect was relatively small compared to the effects of debris flow composition (i.e., coarse-grained fraction, clay fraction, and water fraction) (Figure 11 and 5p–5r). Deposit morphology was unaffected by the outflow plain slope; levee height, lobe height, and lobe width were similar for various outflow plain slopes.

Channel slope varied between 22° and 34° (Figures 12 and 5m–5o). A steeper channel slope led to larger runout and flow velocities, induced by the increased gravitational potential energy. Levee height, lobe height, and lobe width remained nearly constant (Figures 12c and 12e).

Channel width varied between 4.5 and 12 cm (Figures 13 and 5s–5u). Runout distance and area increased with increasing channel width. A smaller channel width probably led to enhanced wall friction and thereby decreased flow velocity (Figure 13b), reducing runout distances. Levee height, lobe height, and lobe width were largely unaffected by channel width (Figures 13c–13e). Most likely, further increasing channel width will eventually result in a decrease in runout distance because of flow thinning.

Increasing the channel length from 2 m to 3 m, while keeping channel slope constant at 28°, had no discernable effect on debris flow velocity and deposits (Figures 14 and 5x). Flow velocity and runout distance, which are linearly related to each other (Figure 14c), were similar for both channel lengths.

3.5. Effects of Flow Momentum on Runout and Deposit Morphology
There is a strong relation between flow momentum, defined as the product of flow velocity and mass, and runout distance and area, regardless of debris flow composition, volume, or terrain geometry.
Figure 15. Relation between flow momentum and runout distance, deposit area, and deposit morphology. (a) Runout distance versus flow momentum. (b) Deposit area versus flow momentum. (c) Lobe height versus flow momentum. (d) Lobe width versus flow momentum. (e) Levee height versus flow momentum. The different symbols correspond to the different series of varied variables shown in Figures 7–14.

(Figures 15a and 15b). Lobe height and width were largely unaffected by flow momentum (Figures 15c and 15d). However, debris flows of low water fraction and debris flows with a large gravel fraction had a relatively low flow momentum and thick lobe. For both the gravel and water fraction series lobe height decreased and flow momentum increased with increasing water fraction or decreasing gravel fraction, respectively (except for very low gravel fractions). For gravel concentration, this trend probably results from the high frontal friction in debris flows with a high coarse-material fraction. Lobe width was large for low-momentum debris flows with low water fraction. When water fraction increased, flow momentum increased and lobe width decreased because of levee formation. In contrast, when water fraction increased further, lobe width increased again despite a further increase in flow momentum. This was caused by a lack of lateral levee formation,

Figure 16. Effects of outflow plain bed. (a, b) Initial loose sand bed of ~1 cm thick: deposit with a thick terminal lobe and well-defined coarse-grained lateral levees and frontal lobe margins. (c, d) Fixed sand bed: large runout, lateral spreading, restricted grain size segregation, and small levees. (e, f) Smooth plastic bed: large runout, lateral spreading, no observable grain size segregation, and no discernable levees.
causing lateral spreading of the debris flow and a wide lobe. Levee height decreased with increasing flow momentum for all series (Figure 15e), because runout distance increased and the same amount of sediment was spread over a longer distance.

3.6. Effects of Outflow Plain Bed Composition

The presence of a loose, erodible, and porous outflow plain bed resulted in the formation of debris flows with well-developed grain size segregation, levees, and a marked depositional lobe as shown in the previous sections. Figure 16 shows three debris flows for which only the outflow plain surface properties varied. In the presence of a loose initial bed of ~1 cm thickness, the debris flow formed an elongated deposit, with a relatively narrow channel bordered by well-developed levees and a relatively thick depositional lobe. Coarse particles were accumulated in the levees and the lobe margins, and there was a clear particle size difference between the deposit margins and interior (Figures 16a and 16b). Debris flows that flowed over a fixed bed of similar composition, which had comparable roughness but whereon erosion and infiltration were impossible, were more mobile than the debris flows on a loose sand bed. Flow spread laterally and only small levees were formed. Grain size segregation was limited, and only a small fraction of coarse particles concentrated in the levees and at the flow front. A weakly developed thin lobe was formed (Figures 16c and 16d). In the presence of a smooth bed that prevented both erosion and infiltration, lateral spreading of the debris flows was even more dramatic, no grain size segregation occurred, and there was no distinction between channel and lobe (Figures 16e and 16f). Mobility of these flows was similar to the mobility on the fixed sand bed, although slightly more mobile on average.

4. Discussion

In this section we discuss effects of debris flow composition on (1) depositional mechanisms, (2) runout, and (3) deposit geometry. We end the discussion with an analysis on the scaling of debris flows in small-scale laboratory flumes.

4.1. Debris Flow Deposition

Debris flow deposition can result from numerous processes: (1) decay of excess pore fluid pressure [e.g., Terzaghi, 1956; Hutchinson, 1986], (2) viscoplastic yield strength [Johnson, 1970; Johnson and Rodine, 1984; Coussot and Proust, 1996], (3) decay of grain collision stresses [e.g., Lowe, 1976; Takahashi, 1978, 1991], and (4) increasing grain contact friction and friction concentrated at flow margins [Major, 1997, 2000; Major and Iverson, 1999]. Support for the various hypotheses has been largely anecdotal, however; only the latter hypothesis is based on in situ measurements from replicable, large-scale flume experiments [Major and Iverson, 1999].

We did not measure load and pore fluid pressure in the runout zone, and therefore, we can only speculate about the processes that caused deposition in our experiments. However, observations of the size of the coarse-grained accumulation at the flow front, along with diffusivity measurements in static mixtures of the investigated flows, suggest that deposition in most of our experimental debris flows is mainly influenced by friction at the frontal flow margins imposed by the accumulation of coarse particles and decay of pore fluid pressure. Escape rates of pore fluid increased with decreasing clay fraction and increasing gravel fraction, as can be visually observed in the supporting information Movies S1–S8. The importance of frontal friction and decay of pore fluid pressure is best illustrated by the relation between gravel fraction and runout (Figures 7a and 7b); a large gravel concentration leads to a large accumulation of coarse particles at the flow front and higher diffusivity (Figure 7g), resulting in early deposition and reduced runout. We hypothesize that frontal friction was most important in the majority of the experimental debris flows (except for those with very high gravel and very low clay concentrations), as we observed retained high pore pressure in most of the deposits’ flow bodies after deposition (water rapidly escaped the deposits upon a slight increase of loading by gently touching the deposit).

In contrast, in the viscous clay-rich experimental debris flows (clay fraction >0.22) the accumulation of coarse sediments at the flow front is marginal to absent and diffusivity of the static mixtures is very low. Yet runout is greatly reduced in these flows (Figures 8a and 8b). We observed no wetting of bed material adjacent to the debris flow after deposition (Movie S5), suggesting retained pore fluid pressures after deposition. These observations suggest that both friction at the flow front and decay of pore fluid pressure did not significantly contribute to deposition in these debris flows. Rather, deposition was probably determined by high effective viscosity and yield strength in these viscous clay-rich debris flows.
4.2. Runout Distance

Trends in debris flow runout were in good agreement with the results obtained in natural flows and other experiments as follows. Similar to empirical relations for natural debris flows [Rickenmann, 1999], our experimental results show that runout strongly depends on flow momentum. Observations of natural debris flows show long runout distances especially when effective lateral levees were formed [e.g., Iverson, 2003; Zanuttigh and Lamberti, 2007], which we also observed in the experiments. However, large accumulations of coarse particles at the frontal flow margins decrease runout distance and area as also found in large experimental debris flows at the USGS flume [e.g., Major and Iverson, 1999]. High clay fractions reduce flow velocity and runout above an optimum value in the experiments. Runout distances and areas become larger for increasing water fractions, as also observed in experimental debris flows of D’Agostino et al. [2010] and Hürlimann et al. [2015]. Additionally, an increase in debris flow volume enhances runout, as often observed for natural debris flows [e.g., Iverson et al., 1998; Zanuttigh and Lamberti, 2007; Griswold and Iverson, 2008]. Larger channel and outflow plain slopes result in larger runout distances and areas, because of the larger gravitational potential energy, in accordance with observations of natural debris flows [e.g., Prochaska et al., 2008]. However, the increase in runout for a similar increase in slope is much larger for the channel slope than for the outflow plain slope. These results show that the response to both compositional and topographic forcings is similar in our small-scale experimental and large-scale natural debris flows.

Our experiments imply that debris flow runout greatly depends on composition. Water content strongly influences runout, and additionally, the amount of clay and relatively large debris (i.e., the particles that accumulate at the frontal flow margins and form levees) strongly influence runout through debris flow composition. The effects of debris flow grain size distribution are barely directly incorporated in current runout distance prediction methods [e.g., Hungr, 1995; Bathurst et al., 1997; Iverson et al., 1998; Rickenmann, 1999; Iverson and Denlinger, 2001; Crosta et al., 2003; Berti and Simoni, 2007; Tang et al., 2012]. In some of the most sophisticated models effects of grain size distribution are incorporated through diffusivity, solid volume fraction, compressibility, and redistribution of pore fluid pressure [Iverson and George, 2014; George and Iverson, 2014], but the effects of grain size segregation and the development of frictional frontal flow margins are not explicitly incorporated in these models. Debris flow composition differs greatly among sites because of different source material and hydrological conditions. As a result, the various approaches to estimate debris flow runout are often site specific [e.g., Fannin and Wise, 2001; Rickenmann, 2005]. Therefore, runout prediction methods, especially empirical-statistical methods, should only be applied to sites with similar conditions to those on which their development is based [e.g., Rickenmann, 2005; Hürlimann et al., 2008]. Nevertheless, also within the same site or fan, where catchment lithology and hydrological conditions are similar, variations in debris flow composition lead to large variability in runout distances [Whipple and Dunne, 1992].

In short, there is no simple and universal runout prediction method, and therefore, there is a need to better understand and describe the depositional characteristics and runout behavior of debris flows [Scheidl and Rickenmann, 2010]. Our results show that significant improvements may be made by incorporating the effects of debris flow grain size distribution, but this requires more observations on experimental and natural debris flows.

4.3. Deposit Morphology

Debris flows occur in many different environments on Earth. They are generated and deposited on hillslopes, catchments, alluvial fans, and channels that can vary greatly in many characteristics, including morphometry and lithology. Additionally, their frequency, initiation mechanism, volume, and composition can greatly vary between sites [e.g., Caine, 1980; Johnson and Sitar, 1990]. Most of these debris flows have similar morphology; they often have a channel bordered by lateral levees and ending in depositional lobes. However, it remains poorly understood what determines the geometry of debris flow deposits.

Debris flow deposit geometry is largely controlled by debris flow composition in our experiments: the coarse-grained, clay, and water fractions all have a profound effect on lobe height, lobe width, and levee height, while the effects of topography and volume are negligible (Figures 7–15). Additionally, outflow bed characteristics influenced deposit geometry (Figure 16). Depositional lobe thickness was mainly determined by the height of the frontal accumulation of coarse particles in our experiments, behind which the more fluidal debris flow body incrementally accreted up to the height of the frontal coarse-particle accumulation. Similar dependence between lobe height and frontal coarse-particle accumulation has been observed in natural debris flows [e.g., Pierson, 1984]. As an exception, in the experimental debris flows wherein clay
fractions exceeded 0.22, high viscosity inhibited grain size segregation and consequently the formation of a coarse frontal accumulation. Yet lobe height slightly increased for higher clay fractions from 0.22 to ∼0.3 (Figure 8). We hypothesize that these clay-rich debris flows behaved as Bingham viscoplastic flows [Johnson, 1970; Coussot et al., 1998], wherein lobe height was mainly determined by an increasing yield strength for higher clay fractions.

4.4. Scaling

Flow behavior, deposit morphology, and particle sorting of our small-scale experimental debris flows were similar to those of natural debris flows, as discussed below. A coarse-grained flow front, followed by more dilute material and formation of lateral levees, observed in our experimental debris flows (Movies S1–S8) is a typical feature of natural debris flows [e.g., Pierson, 1986; Iverson, 1997; Zanuttigh and Lamberti, 2007; Johnson et al., 2012]. The coarse-grained levees and lobe margins found in many of the experimental debris flow deposits are also common in natural debris flow deposits [e.g., Blair and McPherson, 1998; Blair, 1999] (Figure 17). Furthermore, the well-developed grain size sorting implies that the processes that govern the flow behavior of natural debris flows, such as kinematic sorting, squeeze expulsion, and preferential transport of coarse particles to the flow front [Vallance and Savage, 2000; Gray and Kokelaar, 2010; Johnson et al., 2012], were also present in our experimental debris flows. This also implies that frictional forces dominated the flow in the majority of our experimental debris flows [Vallance and Savage, 2000], as they generally also do in natural debris flows [e.g., Zhou and Ng, 2010]. The response of deposit morphology, runout distance, and depositional mechanisms to topographic forcings (i.e., channel slope and outflow plain slope) and internal characteristics (i.e., composition) in our experiments was similar to the response of natural debris flows to these forcings [e.g., Major and Iverson, 1999; Rickenmann, 2005; Zanuttigh and Lamberti, 2007]. These observed similarities between small-scale experimental debris flows and natural debris flows suggest that our small-scale debris flow experiments may efficiently complement field observations to identify many of the controls on natural debris flow behavior and deposits.

Compared to natural debris flows, small-scale experimental debris flows exhibit disproportionately large effects of fluid yield strength, viscous flow resistance, and grain inertia while exhibiting disproportionately
Table 2. Physical and Dimensionless Parameters of Small-Scale, Experimental Debris Flows and Large-Scale, Natural Debris Flows\(^a\)

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Symbol (Unit)</th>
<th>Small-Scale (This Study) Debris Flows</th>
<th>USGS Flume Typical Range Natural Debris Flows(^b)</th>
<th>Typical Range Natural Debris Flows(^b)</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>Physical Parameters</strong></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Typical grain diameter</td>
<td>(\delta) (m)</td>
<td>0.0005–0.002</td>
<td>0.001</td>
<td>10(^{-5})–10</td>
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<tr>
<td>Flow depth</td>
<td>(H) (m)</td>
<td>0.005–0.018</td>
<td>0.1</td>
<td>0.1–10</td>
</tr>
<tr>
<td>Flow velocity</td>
<td>(u) (m/s)</td>
<td>0.9–2.9</td>
<td>10</td>
<td>0.1–20</td>
</tr>
<tr>
<td>Flow shear rate</td>
<td>(\gamma) (1/s)</td>
<td>105–371</td>
<td>100</td>
<td>1–100</td>
</tr>
<tr>
<td>Solid density</td>
<td>(\rho_s) (kg/m(^3))</td>
<td>2650(^h)</td>
<td>2700</td>
<td>2500–3000</td>
</tr>
<tr>
<td>Fluid density</td>
<td>(\rho_f) (kg/m(^3))</td>
<td>1000–1533</td>
<td>1100</td>
<td>1000–1200</td>
</tr>
<tr>
<td>Solid volume fraction</td>
<td>(v_s) (–)</td>
<td>0.35–0.59</td>
<td>0.6</td>
<td>0.4–0.8</td>
</tr>
<tr>
<td>Fluid volume fraction</td>
<td>(v_f) (–)</td>
<td>0.65–0.41</td>
<td>0.4</td>
<td>0.2–0.6</td>
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<tr>
<td>Fluid viscosity</td>
<td>(\mu) (Pas)</td>
<td>0.001–0.0035</td>
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<td>0.001–0.1</td>
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<tr>
<td>Friction angle</td>
<td>(\phi) (deg)</td>
<td>42(^h)</td>
<td>40</td>
<td>25–45</td>
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<tr>
<td>Hydraulic permeability</td>
<td>(k) (m(^2))</td>
<td>1.1 \times 10(^{-16})–2.1 \times 10(^{-13})</td>
<td>10(^{-11})</td>
<td>10(^{-13})–10(^{-9})</td>
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<tr>
<td>Hydraulic diffusivity</td>
<td>(D) (m(^2)/s)</td>
<td>5.8 \times 10(^{-9})–1.2 \times 10(^{-11})</td>
<td>10(^{-4})(^e)</td>
<td>10(^{-8})–10(^{-2})</td>
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<td><strong>Dimensionless Parameters</strong></td>
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<td></td>
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</tr>
<tr>
<td>Savage number</td>
<td>(N_S)</td>
<td>0.17–2.25</td>
<td>0.2</td>
<td>10–7–10(^9)(^b,c,f)</td>
</tr>
<tr>
<td>Bagnold number</td>
<td>(N_B)</td>
<td>37–1589</td>
<td>400</td>
<td>10(^0)–10(^8)(^b,c)</td>
</tr>
<tr>
<td>Friction number</td>
<td>(N_F)</td>
<td>141–2760</td>
<td>2 \times 10(^3)</td>
<td>10(^0)–10(^4)(^b,f)</td>
</tr>
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<td>Mass number</td>
<td>(N_M)</td>
<td>1.2–3.63</td>
<td>4</td>
<td>1–10(^8)</td>
</tr>
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<td>Darcy number</td>
<td>(N_D)</td>
<td>3.2 \times 10(^4)–5.9 \times 10(^7)</td>
<td>600</td>
<td>10(^4)–10(^8)</td>
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<tr>
<td>Grain Reynolds number</td>
<td>(N_{Rg})</td>
<td>31–504</td>
<td>100</td>
<td>0.01–2(^h)</td>
</tr>
<tr>
<td>Reynolds number</td>
<td>(R)</td>
<td>2.3 \times 10(^6)–1.4 \times 10(^5)</td>
<td>3 \times 10(^3)(^d)</td>
<td>10(^5)–10(^3)(^c,d)</td>
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<tr>
<td>Pore pressure number</td>
<td>(N_p)</td>
<td>0.003–200</td>
<td>0.008(^h)6 \times 10(^{-3})(^d)</td>
<td>10(^{-6})–10(^{-1})(^c,d,f)</td>
</tr>
</tbody>
</table>

\(^a\)Values were taken or calculated from the source specified in the header unless specified otherwise in the footnotes.
\(^b\)Iverson [1997].
\(^c\)Iverson and Denlinger [2001].
\(^d\)Iverson et al. [2010].
\(^e\)Major [2000].
\(^f\)Zhou and Ng [2010].
\(^g\)Based on debris flows ranging from 20% to 70% of water by volume.
\(^h\)Estimated values.

little effect of pore fluid pressure (Table 2) [Iverson, 1997; Iverson and Denlinger, 2001; Iverson et al., 2010]. The dimensionless numbers denoting flow dynamics of our experimental debris flows are generally in the range of values that formed in the large-scale USGS flume and of natural debris flows (Table 2) [Iverson, 1997; Iverson and Denlinger, 2001; Zhou and Ng, 2010]. Yet in our experiments the Bagnold, Savage, and grain Reynolds numbers are relatively large, because of shallow flow and high flow velocity, resulting in a high shear rate, and the relatively large characteristic grain size compared to flow depth.

Geometrically, the experimental debris flows are within the range of natural debris flows (Figure 18). We compare debris flow mobility of our experimental debris flows to natural debris flows by comparing total travel distance \(L\) with total elevation difference \(E\) (mobility ratio) and by comparing inundated area with volume. Note that inundated area in Figure 18a incorporates both the planimetric channel area and the planimetric deposit area (in contrast, Figures 7–13 show only deposit area). Similarly, the mobility ratio of the experiments is defined as the total travel distance \(L\) of a debris flow from initiation point (the mixing tank gate) to its farthest point of deposition (lobe terminus) divided by the associated elevation difference \(E\) (rather than the runout distance on the outflow plain as shown in Figures 7–13). The ratio between inundated area and volume of the experimental debris flows is similar to the ratio of natural debris flows (Figure 18a). The best fit regression line for natural debris flows found by Griswold and Iverson [2008] connects the experimental and the natural debris flows, and all experimental debris flows fall within the 99% confidence intervals for prediction.
Figure 18. Comparison between experimental and unconfined and confined natural debris flow dimensions. (a) Area inundated as a function of debris flow volume for the experimental debris flows and natural debris flows (including the USGS flume data) [Griswold and Iverson, 2008]. The solid line is the best fit regression line as reported by Griswold and Iverson [2008] \((A = 20V^{2/3})\), and the dashed lines are the 99% confidence intervals for prediction as given by Griswold and Iverson [2008]. (b) Mobility ratio \((L/E)\), wherein \(L\) is the total travel distance of a debris flow from its initiation point to the farthest point of deposition and \(E\) is the associated elevation difference. Lines denoting the range of natural debris flow mobility ratio based on values reported in Corominas [1996], Iverson [1997], Bathurst et al. [1997], Toyos et al. [2007], and D’Agostino et al. [2010]. (c) Debris flow width to depth ratio \((W/D)\) (width defined as distance between levees and depth as levee height (Figure 2). Data from natural debris flows from Rickenmann [1999], Bulmer et al. [2002], and De Ruig and Hoozemans [1986].

However, most experimental debris flows fall in the lower end of the spectrum, implying that they are relatively short. Similarly, the mobility ratio of the experimental debris flows is at the lower end of the mobility spectrum observed for natural debris flows. For natural debris flows the mobility ratio typically ranges between 1 and 20 [e.g., Corominas, 1996; Iverson, 1997; Bathurst et al., 1997; Toyos et al., 2007; D’Agostino et al., 2010], and Iverson [1997] suggests that \(L/E\) increases logarithmically with increasing volume. The mobility ratio of our experiments ranges from 1.7 to 2.7 (mean = 2.2) (Figure 18b). This suggests that \(L/E\) is particularly small for debris flows of small volume [Iverson, 1997]. The comparison between the mobility of small-scale experimental and natural debris flows is probably unaffected by the dam break initiation of the experimental debris flows, as runout and flow velocity were similar for a 2 and 3 m channel with a 28° slope (Figure 14). As the runout of a debris flow is a function of travel efficiency and expresses energy dissipation both inside and outside the flow [e.g., D’Agostino et al., 2010], we attribute the relatively short runout distance to the disproportional large effects of fluid yield strength and viscous flow resistance, rapid dissipation of pore pressure [Iverson and Denlinger, 2001; Iverson et al., 2010] (Table 2), and the relatively large characteristic grain size compared to flow depth, which together increase the resistance to motion in small-scale debris flows. The width-to-depth ratio of our experimental debris flows fits the range observed in numerous confined and unconfined natural debris flows (Figure 18c).

Many small-scale debris flow experiments have been performed over the past several years, most of which focused on rheology, flow behavior, and the formation of frontal accumulations of coarse debris. Experiments were performed in rotating flumes [Kaitna et al., 2007; Kaitna and Rickenmann, 2007], conveyor belt flumes...
5. Conclusions

We experimentally created small-scale debris flows having self-formed levees and a marked depositional lobe, with flow behavior, deposit morphology, and sediment sorting that were similar to many natural debris flows. The width-to-depth ratio of small-scale experimental debris flows was in the range of natural debris flows. Debris flow runout was also in the range of natural debris flows, but flows were relatively short due to high friction.

Debris flow composition has a profound effect on depositional mechanism, runout, and deposit geometry in our experiments. Debris flow runout increases with an increase in channel slope and width, outflow plain slope, debris flow volume, and water fraction. There is an optimum debris flow composition for maximum runout. Increasing coarse-material fraction increases runout, probably by increased flow confinement by levee formation and grain collisional forces. However, too large coarse-material concentrations cause large frontal accumulations of coarse debris that reduce runout distance, probably by increasing frontal friction together with increased pore fluid pressure decay because of higher diffusivity. An increase in clay fraction enhances runout, most likely because of better retained excess pore pressures. However, too large proportions of clay (>0.22) make debris flows highly viscous so that runout is reduced. Deposition of clay-rich debris flows is likely mainly driven by viscosity and yield strength.

The geometry of debris flow deposits is strongly controlled by debris flow composition: the coarse-grained, clay, and water fractions all have a profound effect on lobe height, lobe width, and levee height. On the other hand, effects of initial conditions of topography (i.e., outflow plain slope, channel slope, and width) and volume are negligible in our experiments.

A loose erodible bed enabling infiltration was essential for the formation of well-developed self-formed levees and grain size segregation in our small-scale experiments. This may explain the absence of these features in previous small-scale experiments.

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